PROPOSED TECHNIQUE THAT SIMULATES THE ACTUAL ASPHALT MIX FIELD COMPACTION

طريقة مقترحة لدمك الخلطات الأسفلتية تماثل الدمك الحقلى

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الملخص العربي

تحتاج صناعة الخرسانة الأسفلتية إلى تحديث وتطوير لمواصفات التصميم والتنفيذ لضمان أفضل أداء للرصف لمواكبة التحديات المستقبلية في هذا المجال، وتعتبر عملية فرش ودمك الخلطات الأسفلتية عملاً مهما أثناء الإنشاء لأن طريقة الدمك في الموقع تؤثر تأثيرا ملحوظا على خصائص وأداء الرصف بالإضافة إلى عمره الزمني. ويتم الدمك في الموقع باسلوب لا يتوافق مع ذلك الذي يتم في المعمل عند التصميم بطريقة مارشال وبالتالي فإن التنبو بسلوك وخصائص الخلطة الأسفلتية المجهزة لاختبار مارشال قد لا يتوافق مع واقع الدمك الحقلي. وقد حاولت العديد من الدراسات السابقة استخدام أساليب دمك معملية مختلفة لمحاولة محاكاة ما يحدث في الطبيعة لكي يتم التنبؤ بالسلوك الفعلي للخلطة تحت تأثير الأحمال المرورية.

وتهدف هذه الدراسة إلى تقديم طريقة دمك معملية جديدة تحاكى ما يحدث أثناء دمك الخلطة الأسفائية فى الطبيعة وذلك باستعمال جهاز مارشال - CBR (نسبة تحمل كاليفورنيا) العادى. ويتم فى تلك الطريقة المقترحة وضع قرص حديدى أعلى العينة فى قالب مارشال ثم الضغط بانتظام لحين الحصول على أبعاد العينة القياسية. وعلى ذلك فقد تم إختيار عشر خلطات مختلفة من طرق يتم تنفيذها بواسطة الهينة العامة للطرق والكبارى، وتم استخدام ثلاثة اساليب دمك مختلفة وحساب خصائص الخلطات الناتجة عن تلك الأساليب. الأسلوب الأول وهو استخدام طريقة مارشال التقليدية لتصميم الخلطة والأسلوب الثانى هو نفس طريقة مارشال التقليدية مع إختلاف فقط فى أسلوب دمك العينة حيث تم إستخدام أسلوب الدمك المفترح لمحاكاة ما يحدث فى الطبيعة. أما خصائص الخلطة الأسفلتية التى تم دمكها فى الموقع باستخدام الأسلوب الثالث (طريقة الدمك العادية فى الموقع) فقد تم أخذ عينات من الخلطات المدموكة حديثا فى الطبيعة بجهاز الكور لنفس التصميمات المعابقة، حيث تم إعتبار خصائص تلك الخلطات فى الطبيعة هى المرجع فى مقارنة نتائج أسلوبي الدمك المعملى.

وقد أظهر تحليل النتائج أن طريقة الدمك المعملي المقترحة تحقق خصائص تكاد تكون متطابقة مع الخصائص الفعلية في الموقع لكل الخلطات التي تم اختبارها. أما طريقة مارشال التقليدية فهي تعطى خصائص غير معبرة بدرجات متفاونة عن الخصائص الفعلية في الواقع.

ABSTRACT

Future needs and challenges in the asphalt concrete industry should make use of developed and new techniques to attain the best field performance. Spreading and compacting fresh asphalt concrete mix is a very important job during pavement construction. The compaction mechanism in the field greatly affects the pavement characteristics, performance, and service life. Several studies tried to use different compaction techniques to simulate the actual field compaction in the laboratory. The purpose of this study aims at introducing an innovative compaction technique that simulates the actual field compaction conditions to a great extent as presented in the research results. The technique employs the traditional Marshall-CBR testing machine in compacting the asphalt mixes. A steel disc is placed over the sample, which is continuously compressed in the Marshall mould till reaching the required sample dimensions. Results obtained from the traditional Marshall and the proposed compaction techniques are compared with field results. Analysis of the results showed large coincidence between the proposed compaction technique and the field results. On the other hand, specimens compacted using the traditional Marshall technique showed larger deviation with the field results in almost all the tested properties.

KEW WORDS: Asphalt concrete mix, Marshall test, Compaction technique, Mix unit weight, Mix stability, and Air voids.

4.1 Analysis of Unit Weight Results

The field collected samples are used to obtain the asphalt mix unit weight according to ASTM D2937-04 [11]. Table (5) presented the results of unit weights the obtained from two laboratory techniques; traditional Marshall (yM), and the compaction simulation (y_s), as well as that obtained from the field and denoted by y_6 . Table (5) shows ratios of unit weights obtained by the traditional Marshall technique with the field unit weights (γ_M/γ_f). Somewhat large differences are noticed in this case with the unit weights ratio ranged from 101.53% up to 106.95%. On the other hand, great coincidence is noticed between the values obtained by the proposed compaction technique and those field values, ranging from 99.86% to 100.17%. It is also noted that the average $\gamma_{\rm M}/\gamma_{\rm f}$ was 103.80 and $\gamma_{\rm s}/\gamma_{\rm f}$ was 100.02, (with standard deviation of 2.22 and 0.11 for the two ratios respectively). The statistical results assured that proposed the compaction technique matches to the maximum extent the actual field compaction process.

Figure (2) represents the relation between field density (γ_f) and Marshall unit weight (γ_M). It is shown from the figure that the correlation coefficient (R^2) between the two parameters is 0.667, compared with R^2 = 0.999 which is the correlation coefficient between field unit weight (γ_f) and simulated one (γ_s), as shown in Figure (3). These results certify that the field unit weight has higher correlation with the simulated unit weight, and lower one with the Marshall compaction technique.

4.2 Analysis of Stability Results

The effect of the compaction technique on the asphalt mix stability is shown in Table (6). Based on the tabulated stability values, it can be noted that the compaction technique greatly affects the stability values. However, the mixes compacted using traditional Marshall compaction technique (hammering) has higher stability values than

compacted by the simulated technique (quasi-static) and field samples. This result indicated that the stability values are usually over-estimated when compacting by hammering, but in reality the actual (field) values are usually lower. The obtained values ranged from 102.51% up to 122.84%. One more time, the proposed compaction technique gave closer results of the stability numbers giving values ranging from 99.45% to 101.23% only. However, the mean value for Stab_M/Stab_l was 113.84% and the mean value for Stabs/Stabs was only 100.27%. Moreover, the standard deviation was 6.70 and 0.69 for the results of the two techniques respectively.

Profound analysis of stability data shows that, Marshall stability has higher values than that of simulated and field values by about 270 lbs in the average. This means that, the field stability values are decreased by about 14% in the average than that of Marshall values. This in turn means that the design is performed on high stability values, while in the field these values are never reached leading to premature failure of asphalt pavements. In addition, two mixes namely 1 and 8, as presented in Table (6), are in reality out of specifications giving acceptable stability values using traditional Marshall technique. Therefore, the lower limit of the accepted stability values as obtained from traditional Marshall technique should be elevated to reach 2070 lbs instead of 1800 lb [12] to assure successful field stability values.

Figure (4) shows the relation between field stability and Marshall stability with correlation coefficient, $R^2 = 0.77$. Figure (5) illustrates the relation of field stability versus simulated stability was $R^2 = 0.99$.

4.3 Analysis of Air Voids Results

Table (7) shows the results of the air voids for the different tested specimens compacted using the different compaction techniques for the binder and surface courses. These mixes are classified into two groups, the first group is concerned with

binder course mixes (No. 1, 3, 5, 7, 8 and 9). The second group is related to surface course mixes (No. 2, 4, 6 and 10). Based on the data illustrated in the table it can be noticed that, the traditional compaction procedure involved in Marshall test achieve lower air voids than that of obtained by both the simulated compaction technique and the actual field compaction. The voids ratio in this case A.V_M/A.V_f ranged from 76.74% up to 92.11% indicating larger field voids than that of the traditional Marshall technique (with mean value of 82.98% and standard deviation of 5.04). This is mainly due to a simple fact, compaction by hammering gave slightly higher unit weight, but noticeably lower voids ratio. However, the ratio of A.V₅/A.V_f ranged from 96.08% up to 105.26% indicating closer representation of the proposed compaction technique to the actual field compaction technique. Moreover, the mean value was 99.35% and the standard deviation was 2.99.

Increasing the air voids content in either the binder or surface courses may lead to aging or oxidation and consequently decreasing flexibility that result in raveling. In addition, increasing the air voids may cause penetration of water in the underneath courses which may cause swelling and in turn pavement cracking. This situation can be obviously noticed in mix number 2 in which the field air voids is 5.2%, exceeding the upper limit of 5% for surface course layer [12], despite giving an acceptable value of 4.2% from traditional Marshall technique. Based on air voids analysis it can be concluded that the simulated compaction technique has higher correlation with the field air voids than Marshall technique. Therefore, simulated compaction technique can be considered representative and realistic than Marshall technique. It is worth mentioning that the average value of field air voids are greater than that of common Marshall method and simulated technique by 0.774 and 0.04 respectively. Based on this result, it can be suggested that the specifications

must limit the acceptable air voids percent up to 7.2% and 4.2% for binder and surface course layers respectively, instead of the currently used values of 8.0% and 5.0% [12] when using traditional Marshall technique.

Figure (6) shows the relation between air voids in the field and the corresponding design values obtained by Marshall test. The correlation coefficient for this relation is $R^2 = 0.82$ compared with $R^2 = 0.97$ which is the correlation coefficient for the relation between field and simulated air voids, as shown in Figure (7).

4.4 Analysis of Flow Results

The flow values presented in Table (8) shows that, both the simulated and field techniques gave higher mix flow values than those obtained using Marshall technique. Although all the flow values are within the acceptable range, the values obtained by the simulated technique are pronouncedly closer to the field values. The ratio of M.F_M/M.F_f is ranging from 86.61% up to 99.21% with an average value of 93.70% and standard deviation of 3.95. On the other hand, the ratio of M.F./M.F. is ranging from 98.15% up to 101.89% with an average value of 100.17% and standard deviation of 1.12. Observation of the results showed very close representation of the simulated technique to field compaction.

The correlation coefficient $R^2 = 0.87$ for the relation between field and Marshall flow values is shown in Figure (8), whereas $R^2 = 0.99$ for field and simulated flow values as illustrated in Figure (9). The higher flow values decrease the pavement capability to sustain traffic loads and environmental conditions.

5. CONCLUSIONS

Referring to the analysis of the study results, the following conclusions can be obtained:

1- The unit weights of the specimens compacted using the proposed

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compaction simulation technique are almost identical with those values obtained from the field, with almost perfect correlation (R² = 0.999). Although higher unit weights are obtained when using the traditional Marshall compaction technique but the judgment in this case should be based on the actual field values, with low correlation coefficient equals 0.667.

- 2- Lower stability values are obtained using the simulated technique compared to those of the traditional Marshall procedure. However, field stability values are approximately the same as those of the simulated compaction technique with high correlation coefficient ($R^2 = 0.99$). On the other hand, lower correlation was found between field and Marshall stability values ($R^2 = 0.77$).
- 3- Marshall stability has an average of about 270 lbs higher than that of the proposed compaction simulation technique and field samples. This may explain the premature failure of some asphalt pavements.
- 4- Air voids obtained from both field and proposed technique samples are higher than that obtained with common Marshall test. A strong correlation is found between field and simulated air voids values with (R² = 0.97) compared by the correlation between field and Marshall values with (R² = 0.83).
- 5- The average air void values in the field are greater than that of the common Marshall test procedure and simulated procedure by about 0.8% and 0.04% respectively.
- 6- The compaction simulation technique has approximately the same flow values obtained from field samples ($R^2 = 0.99$), whereas the common Marshall test achieved lower and different flow values when compared with field values ($R^2 = 0.87$).
- 7- Despite giving better properties in all cases, compaction using the traditional Marshall technique could not ever be

achieved in the field. Therefore, the proposed laboratory compaction simulation technique is much more relevant to the actual field compaction process.

6. RECOMMENDATIONS

Based the study analysis and conclusion the following can be presented:

- 1- It is recommended to use the proposed compaction simulation technique for asphalt concrete mix compaction instead of common hammering technique when designing asphalt mixes using Marshall procedure.
- 2- If the traditional Marshall compaction technique is used in the asphalt mix compaction, the accepted values of mix stability should not less than 2070 lbs to ensure best pavement performance in the field.
- 3- When using the traditional Marshall technique for asphalt mix compaction the permitted air voids percent must not be more than 7.2% for binder and 4.2% for surface courses respectively.
- 4- A Comprehensive experimental testing program is recommended to study the effect of the proposed compaction simulation technique on the asphalt mix properties under several mix conditions, testing procedures, and larger number of mixes.
- 5- Advanced testing equipments including new compaction techniques must be introduced in Egyptian laboratories to increase the reliability of the testing and consequently design procedures.

7. REFERENCES:

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